

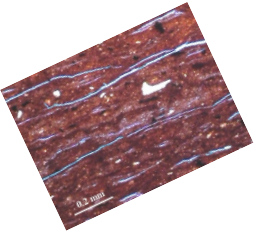


Mechanical Anisotropies of Shales

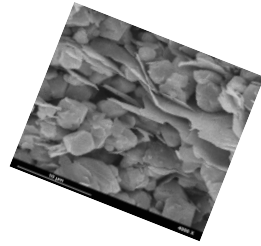
Rune M Holt, NTNU & SINTEF

With contributions from Andreas Bauer & Erling Fjær (SINTEF & NTNU), Per Horsrud (Statoil), Olav-Magnar Nes (Det Norske Oljeselskap) & Jørn F Stenebråten (SINTEF)



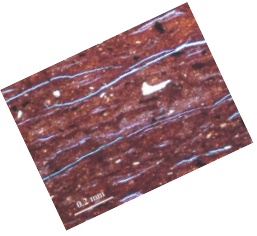


Shales are anisotropic...

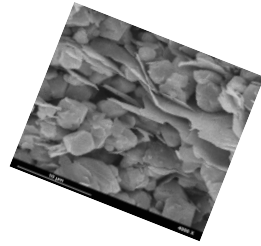


- **Seismic Anisotropy**
 - Influence on AVO, Travel times, Shear Wave Splitting
 - Thomsen-parameters ϵ , γ and δ

- **Rock Mechanical Anisotropy**
 - We are aware...
 - .. but do we care... ?



Shales are anisotropic...



- **Dynamic Anisotropy**
 - Wave velocity anisotropy can be measured quickly in the lab
 - In principle, only one core needed
- **Static Anisotropy**
 - Time consuming tests
 - Several cores needed
- **Static \neq Dynamic moduli;
What about their anisotropies?**

Dynamic moduli

- We determine directly 4 elastic coefficients from ultrasonic measurements (+ Thomsen's ε and γ):

$$C_{11} = \rho v_{Pr}^2, \quad C_{33} = \rho v_{Pz}^2$$

$$\varepsilon = \frac{C_{11} - C_{33}}{2C_{33}}$$

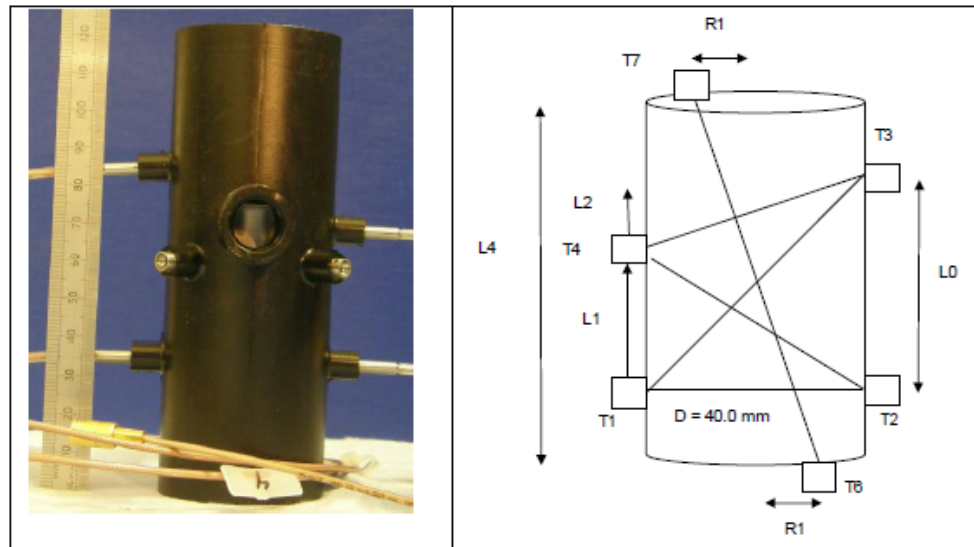
$$C_{44} = \rho v_{Sz}^2, \quad C_{66} = \rho v_{Sr}^2$$

$$\gamma = \frac{C_{66} - C_{44}}{2C_{44}}$$

- From the angular dependence of v_p we find C_{13} (=>Thomsen's δ):

$$\delta = \frac{(C_{13} + C_{44})^2 - (C_{33} - C_{44})^2}{2C_{33}(C_{33} - C_{44})}$$

$$C_{13} = (C_{33} - C_{44}) \sqrt{1 + \frac{2C_{33}\delta}{C_{33} - C_{44}}} - C_{44}$$



Static Moduli

- Static moduli: Slopes of stress vs. strain curves
- Different moduli require different stress paths, e.g.
 - Uniaxial strain \perp symmetry plane (|| symmetry (z-)) axis:

$$\left(\frac{\Delta\sigma_z}{\Delta\varepsilon_z} \right)_{\varepsilon_r=0} = C_{33} \qquad \left(\frac{\Delta\sigma_r}{\Delta\varepsilon_z} \right)_{\varepsilon_r=0} = C_{13}$$

- Uniaxial stress ("triaxial") tests:

- Load || symmetry (z-) axis $\left(\frac{\Delta\sigma_z}{\Delta\varepsilon_z} \right)_{\Delta\sigma_r=0} = E_{\perp} = \frac{[C_{33}(C_{11} - C_{66}) - C_{13}^2]}{(C_{11} - C_{66})}$
- Load \perp symmetry (z-) axis $\left(\frac{\Delta\sigma_z}{\Delta\varepsilon_z} \right)_{\Delta\sigma_r=0} = E_P = \frac{4C_{66}[C_{33}(C_{11} - C_{66}) - C_{13}^2]}{(C_{11}C_{33} - C_{13}^2)}$

- Constant mean stress (z is symmetry axis):

$$\left(\frac{\Delta\sigma_z}{\Delta\varepsilon_z} \right)_{\Delta\bar{\sigma}=0} = f(C_{11}, C_{33}, C_{66}, C_{13}) \rightarrow 2C_{44} \quad (\text{weak anisotropy})$$

Bounds on Transversely Isotropic elastic moduli

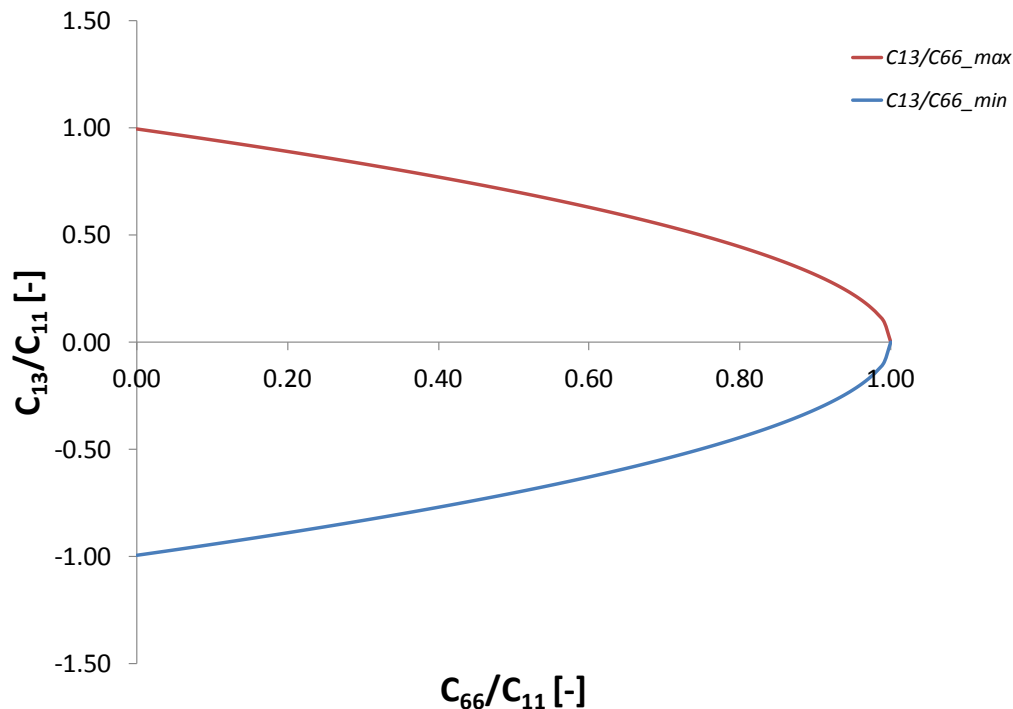
- Positive definiteness of elastic energy \rightarrow

$$C_{44} \ \& \ C_{66} > 0$$

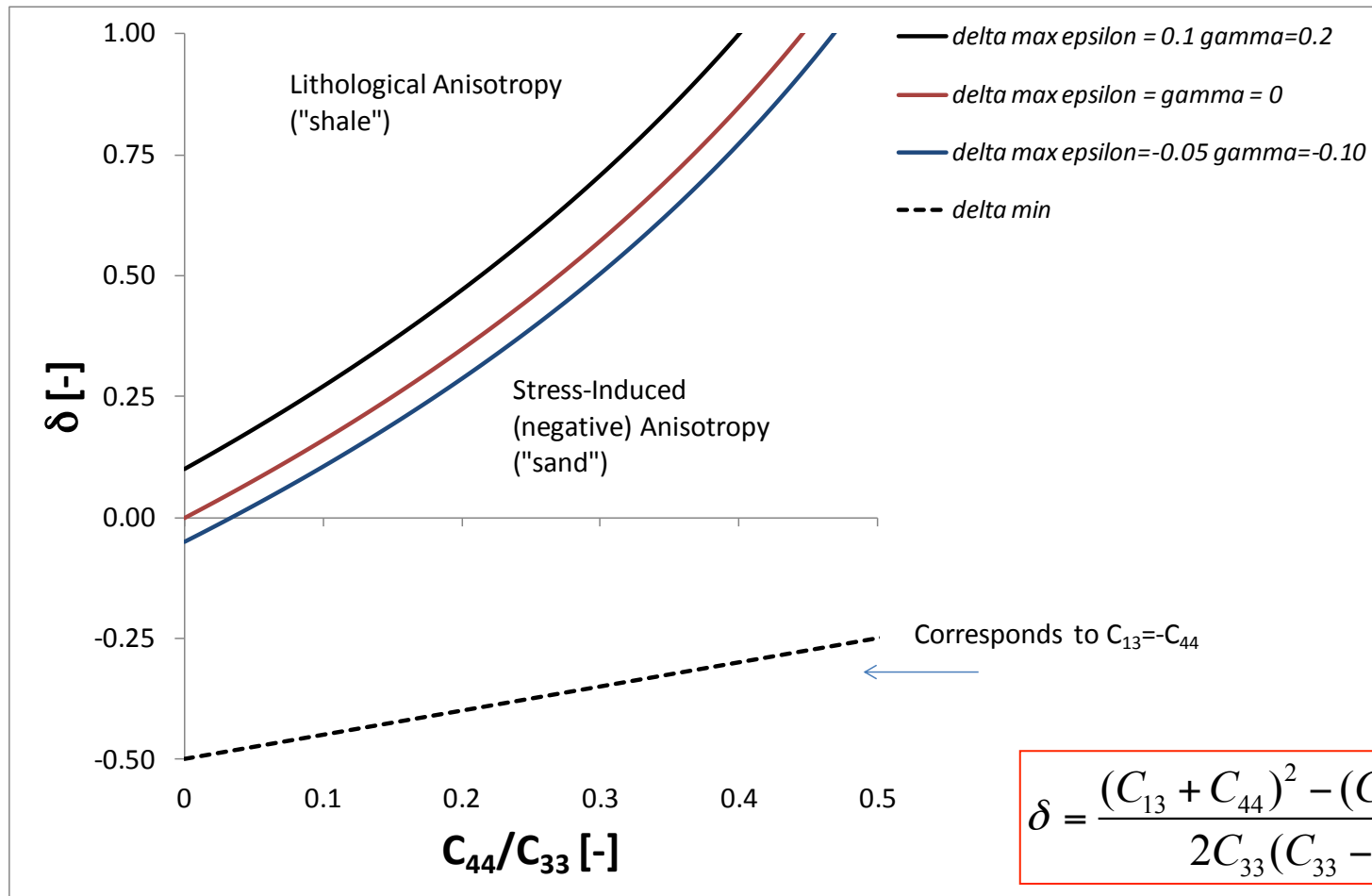
$$E_{\perp} \ \& \ E_p > 0 \Rightarrow C_{11} > C_{66}$$

$$(C_{11} - C_{66})C_{33} - C_{13}^2 > 0$$

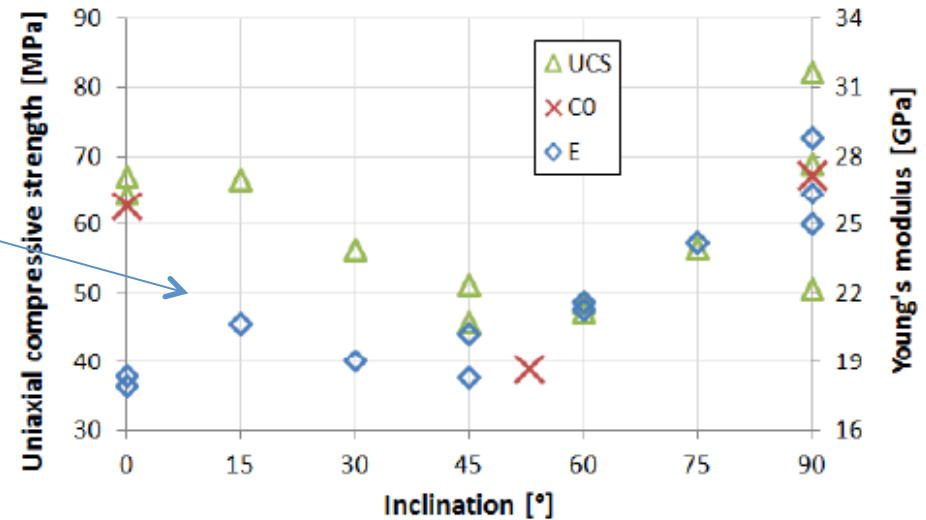
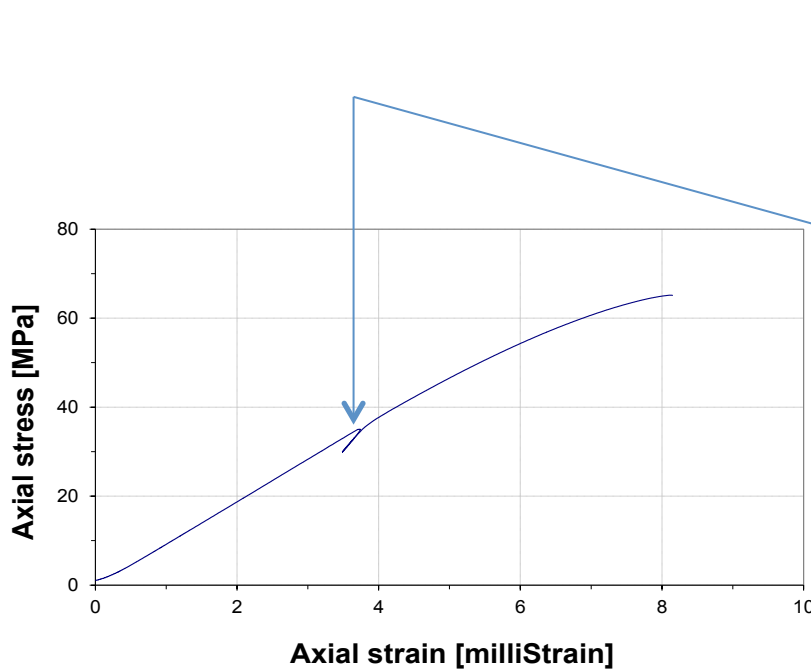
$$\Leftrightarrow -\sqrt{(C_{11} - C_{66})C_{33}} < C_{13} < \sqrt{(C_{11} - C_{66})C_{33}}$$



Bounds on Thomsen's δ



Static Mechanical Anisotropies; Mancos Shale

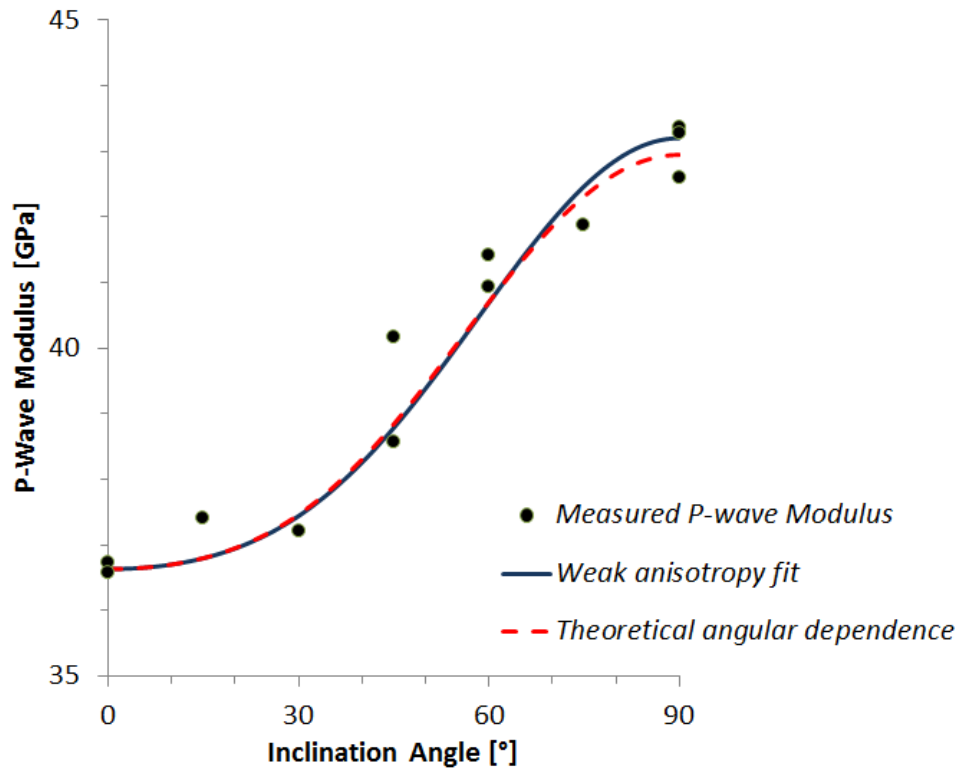


Nes & Fjær, ARMA 2013

- Idea of "patchy" plane of weakness may explain observed strength anisotropy

8% porosity, 24% clay,
unsaturated

Mancos Shale: Ultrasonic P-Wave Anisotropy



Thomsen parameters
used in fit:

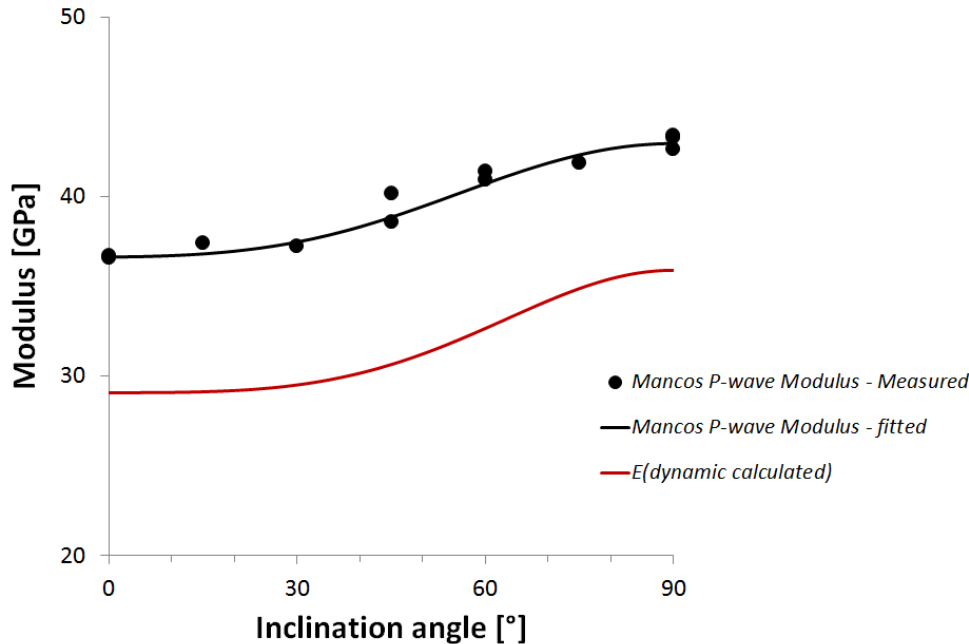
$$\varepsilon = 0.086$$

$$\gamma = 0.152$$

$$\delta = 0.03$$

- Velocity measured under unconfined conditions using several core plugs

Mancos Shale: E-Modulus Anisotropy



- E-modulus predicted from ultrasonic velocities, accounting for observed P- and S-wave velocity anisotropy

$$\frac{1}{E(\theta)} = \frac{1}{E_{\perp}} \cos^4 \theta + \frac{1}{E_P} \sin^4 \theta + \frac{1}{B} \sin^2 \theta \cos^2 \theta$$

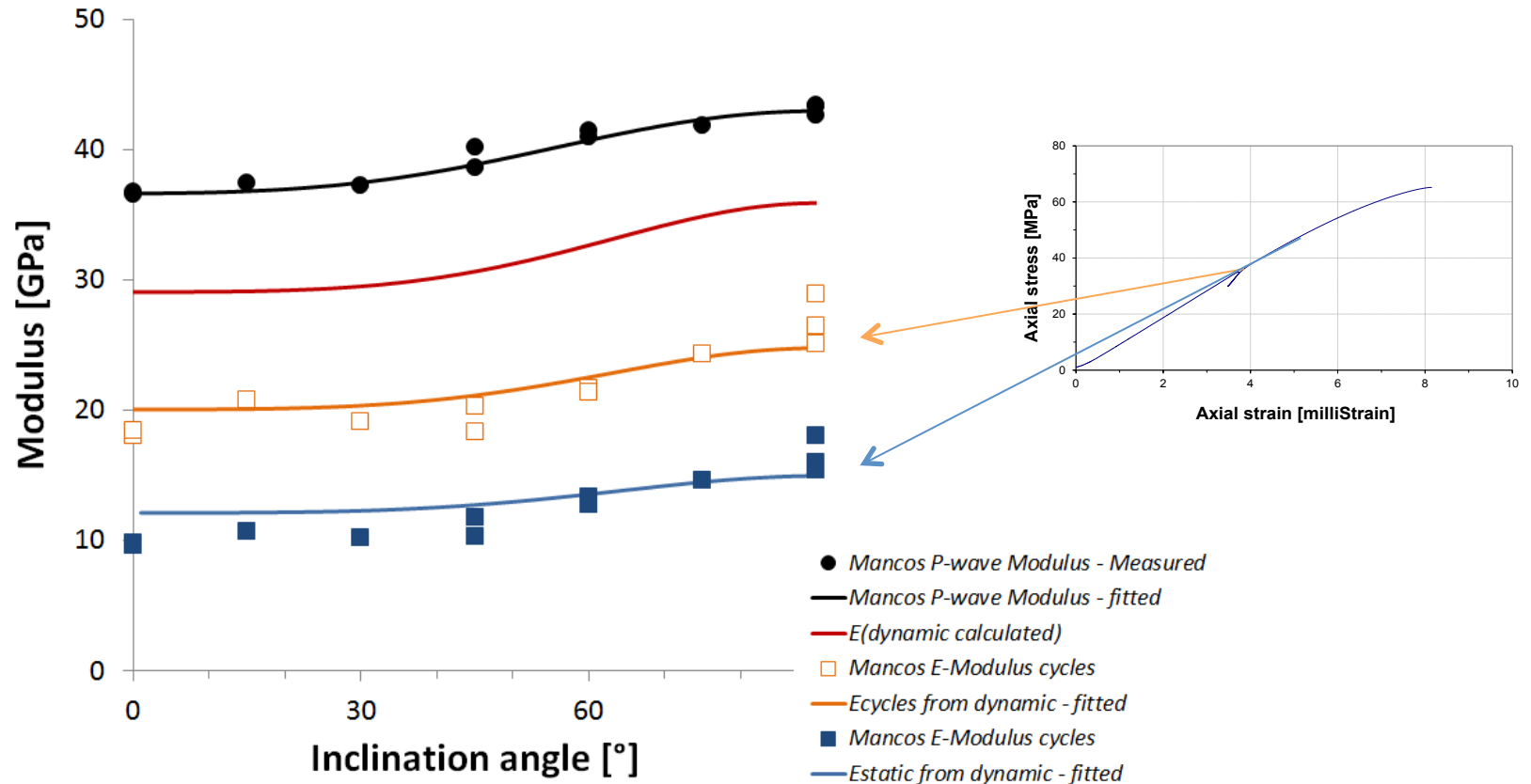
$$E_{\perp} = \frac{[C_{33}(C_{11} - C_{66}) - C_{13}^2]}{(C_{11} - C_{66})} \approx \frac{C_{44}(3C_{33} - 4C_{44})}{(C_{33} - C_{44})} [f_{\perp}(C_{IJ}, \varepsilon, \gamma, \delta)]$$

$$E_P = \frac{4C_{66}[C_{33}(C_{11} - C_{66}) - C_{13}^2]}{(C_{11}C_{33} - C_{13}^2)} \approx \frac{C_{44}(3C_{33} - 4C_{44})}{(C_{33} - C_{44})} [f_P(C_{IJ}, \varepsilon, \gamma, \delta)]$$

$B = \text{etc...}$

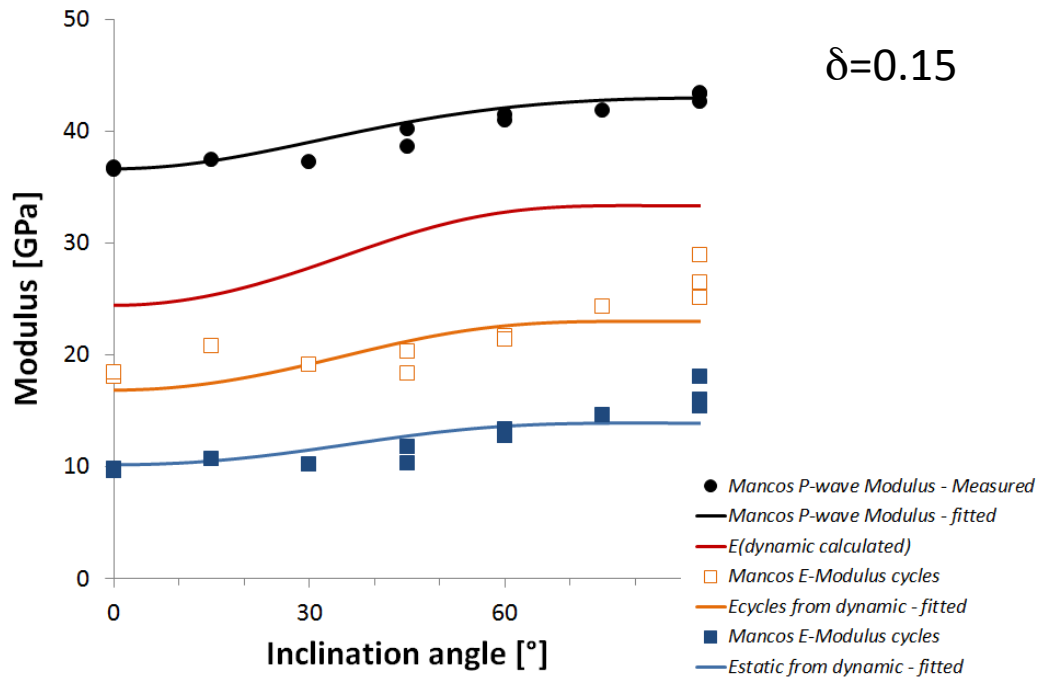
Same parameters used in fit for P-wave modulus and E

Mancos Shale: E-Modulus Anisotropy



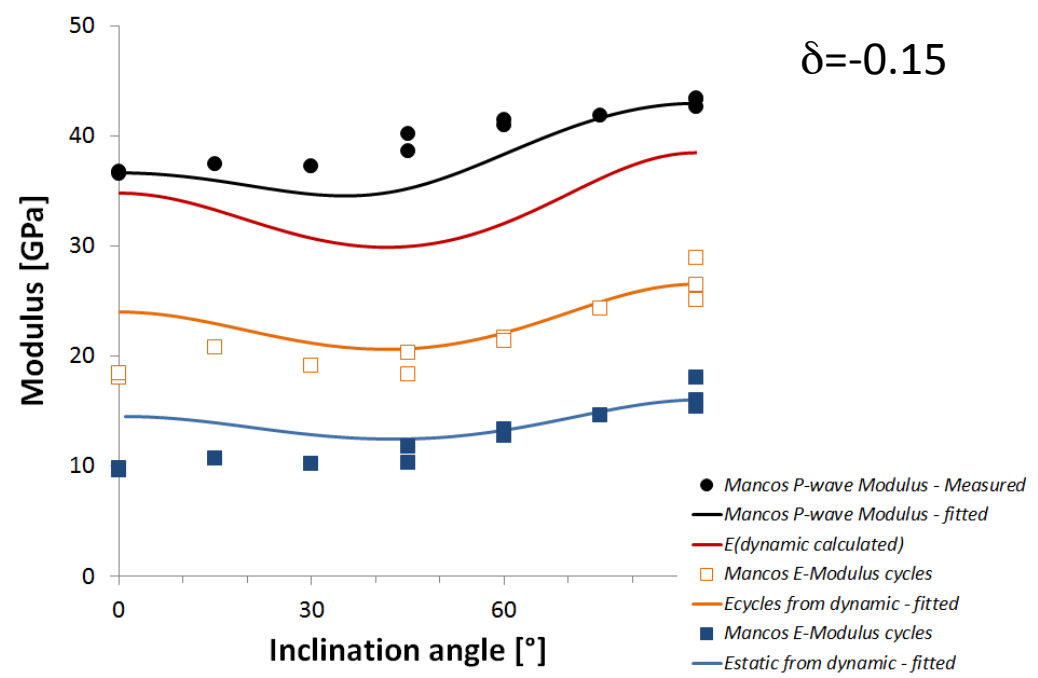
Same parameters used in fit for P-wave modulus and E

- E-modulus predicted from ultrasonic velocities, accounting for observed P- and S-wave velocity anisotropy, and reduced by a factor 1.45 to fit the measured E in cycles, and 2.5 to fit the E measured in 1st time loading



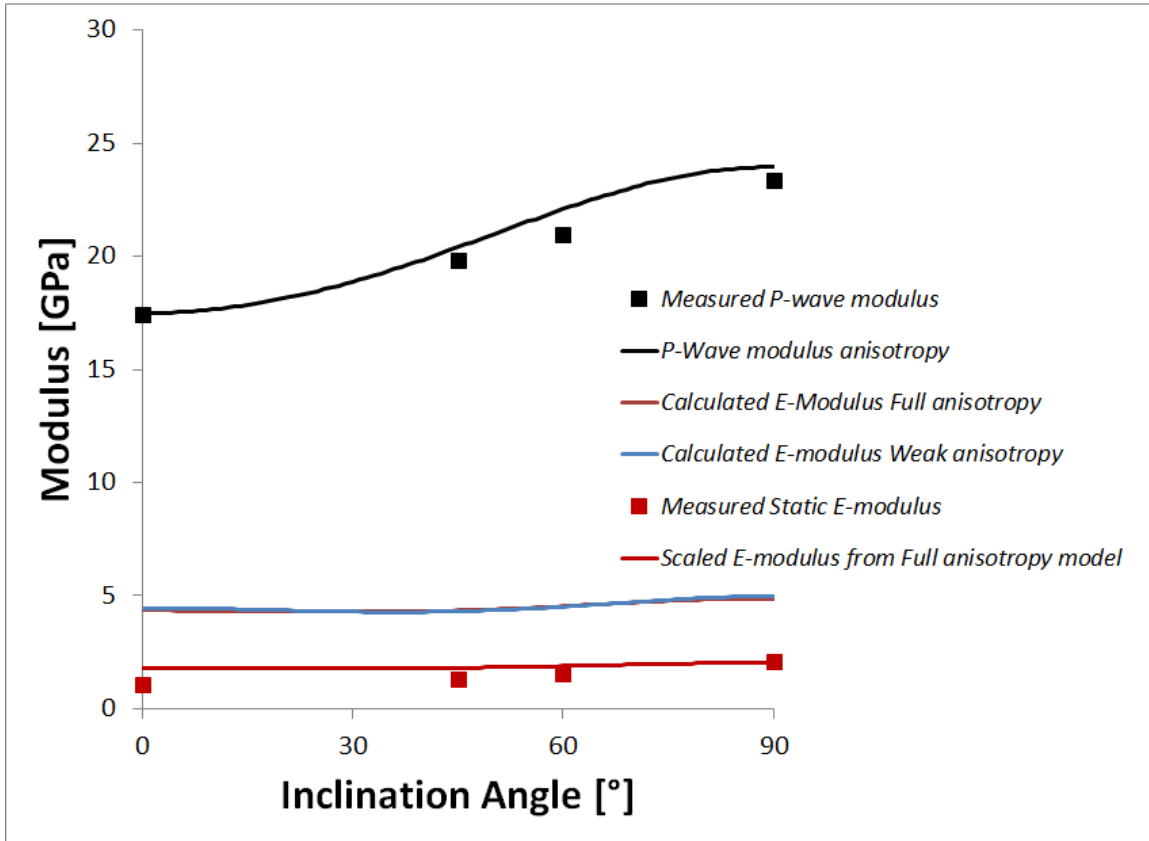
$\delta=0.15$

- Sensitivity of angular dependence to C13 (δ); Mancos Shale



$\delta=-0.15$

Pierre Shale



Thomsen parameters
used in fit:

$$\varepsilon = 0.17$$

$$\gamma = 0.02$$

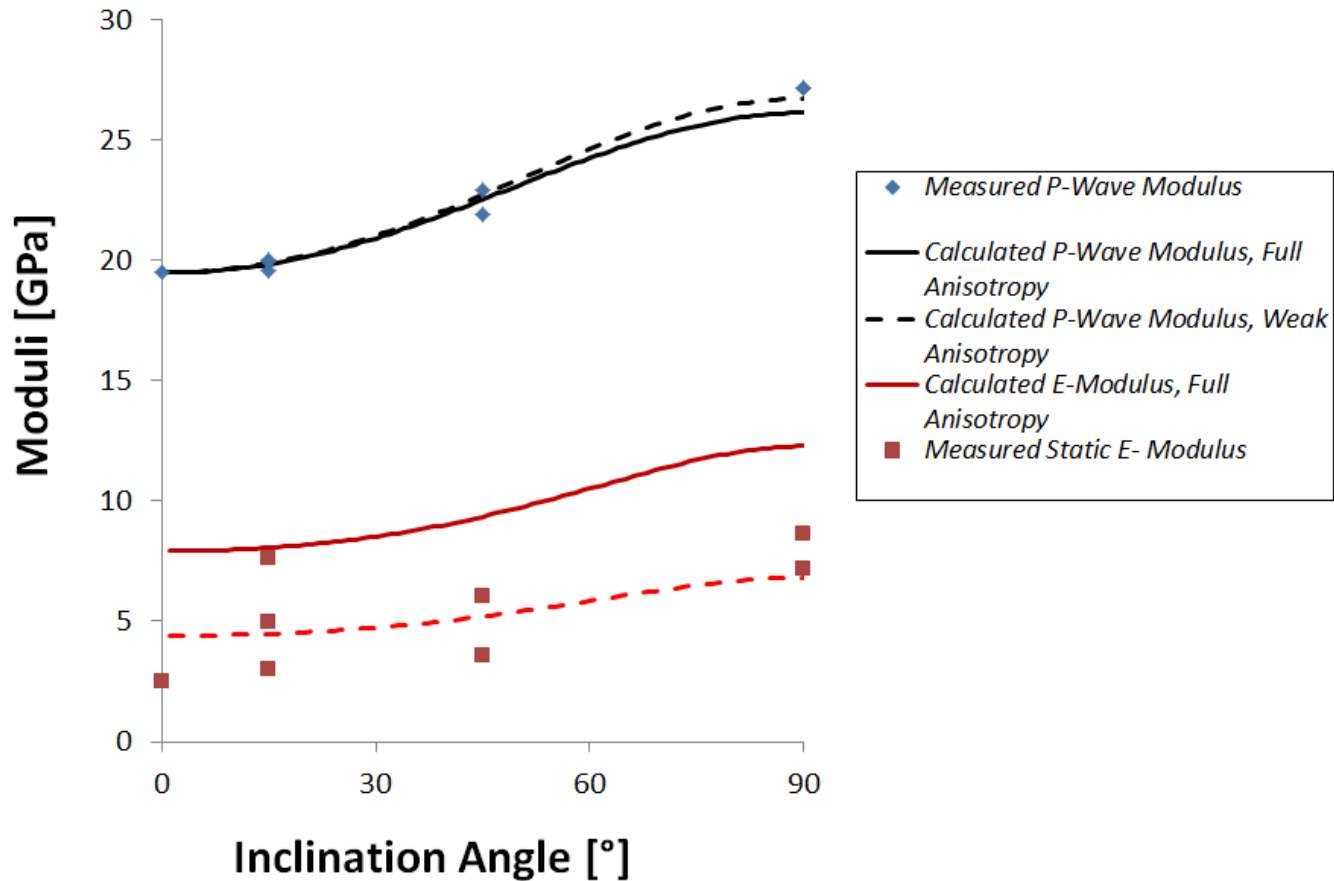
$$\delta = 0.15$$

Dynamic / Static
scaling factor: 2.4

- 25 MPa confining pressure, 10 MPa pore pressure

19-26% porosity, 40-60% clay,
brine saturated

Field Shale



Thomsen parameters
used in fit:

$$\varepsilon = 0.171$$

$$\gamma = 0.235$$

$$\delta = 0.150$$

Dynamic / Static
scaling factor: 1.8

These shales have

15% porosity, 50-60% clay,
brine-saturated

Conversion between Static & Dynamic Anisotropy

- Moduli are very different, but the angular dependence appears within measurement uncertainty to be described by the dynamic anisotropy parameters

Why are Static \neq Dynamic Moduli?

Fluid contribution

Static
drained (normally)

Dynamic
undrained (always)

$$K_{fr} \quad \Leftrightarrow \quad K_{fr} + K_f \frac{\alpha^2}{\phi + \frac{K_f}{K_s} (\alpha - \phi)}$$

Negligible for gas saturation

Dispersion

Ultrasonic: $f \sim 1$ MHz

Sonic: $f \sim 10$ kHz

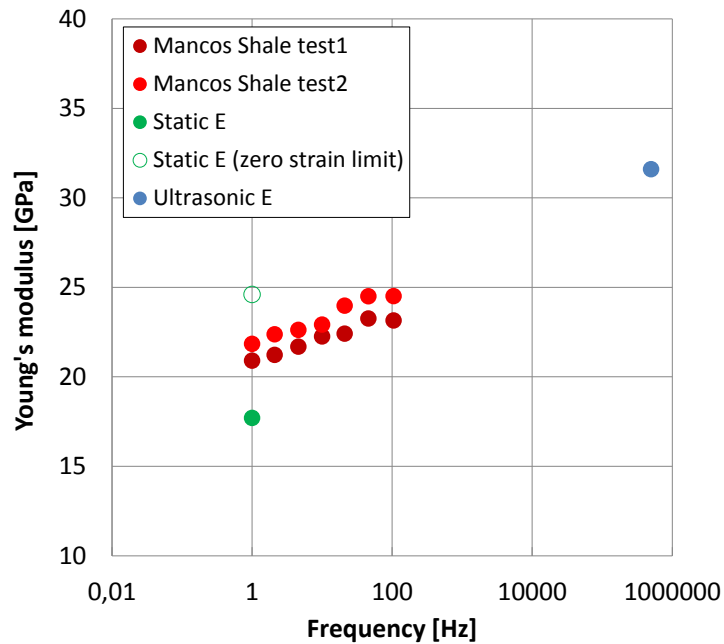
Static: $f \sim 1$ Hz

Plasticity

Static moduli are measured at finite strains and include elastic + plastic deformation; Dynamic moduli are measured at infinitesimal strain and are hence purely elastic.

+ Scale effects, Anisotropy, a.o.

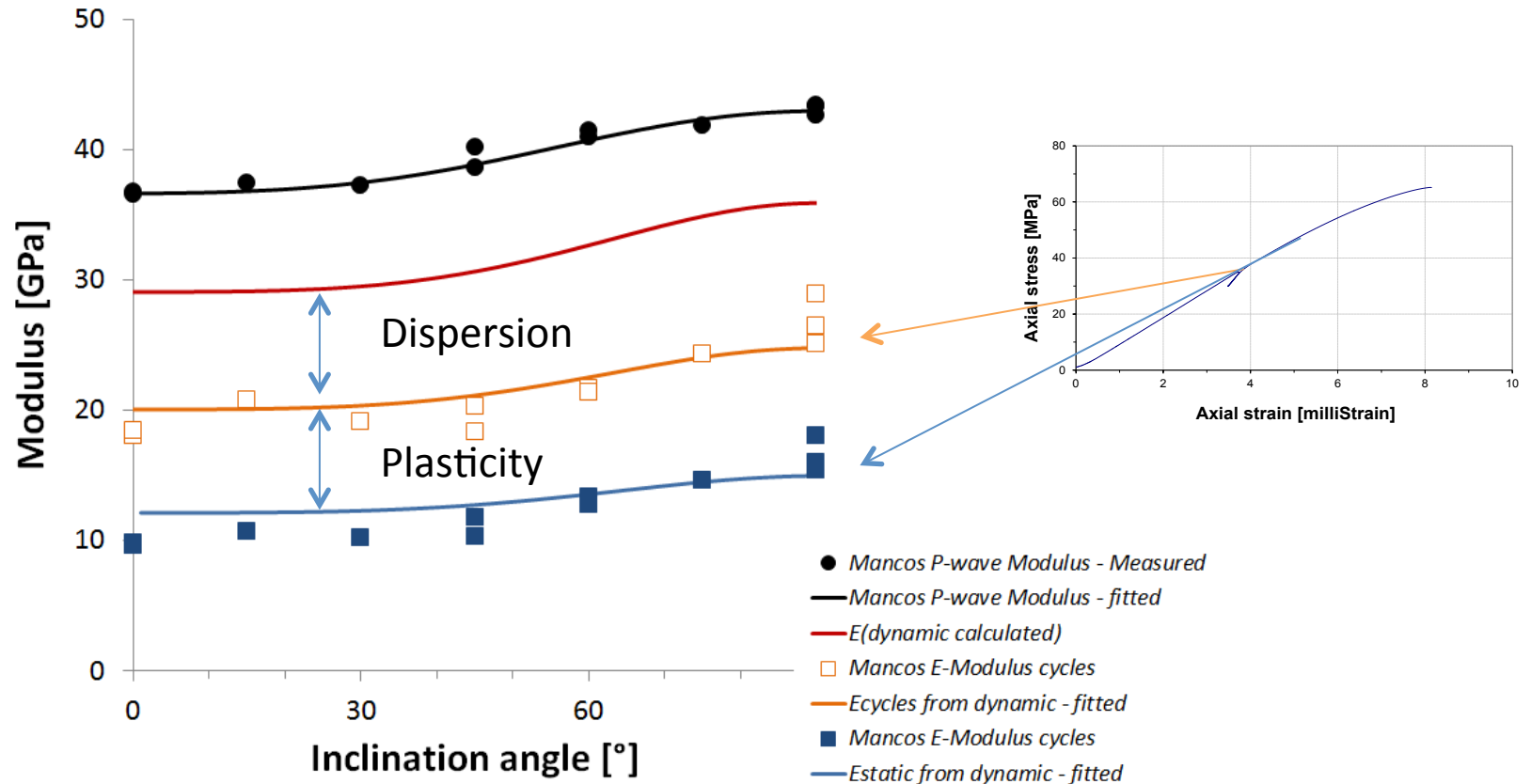
Dispersion in Mancos Shale



Experiment
performed
@ ambient
conditions

- Ultrasonic : Seismic Modulus ~ 1.5
- A part of static vs dynamic discrepancy in Mancos Shale stems from dispersion
- Possible mechanisms: Viscous shear relaxation / Bound water effect or Patchy saturation?

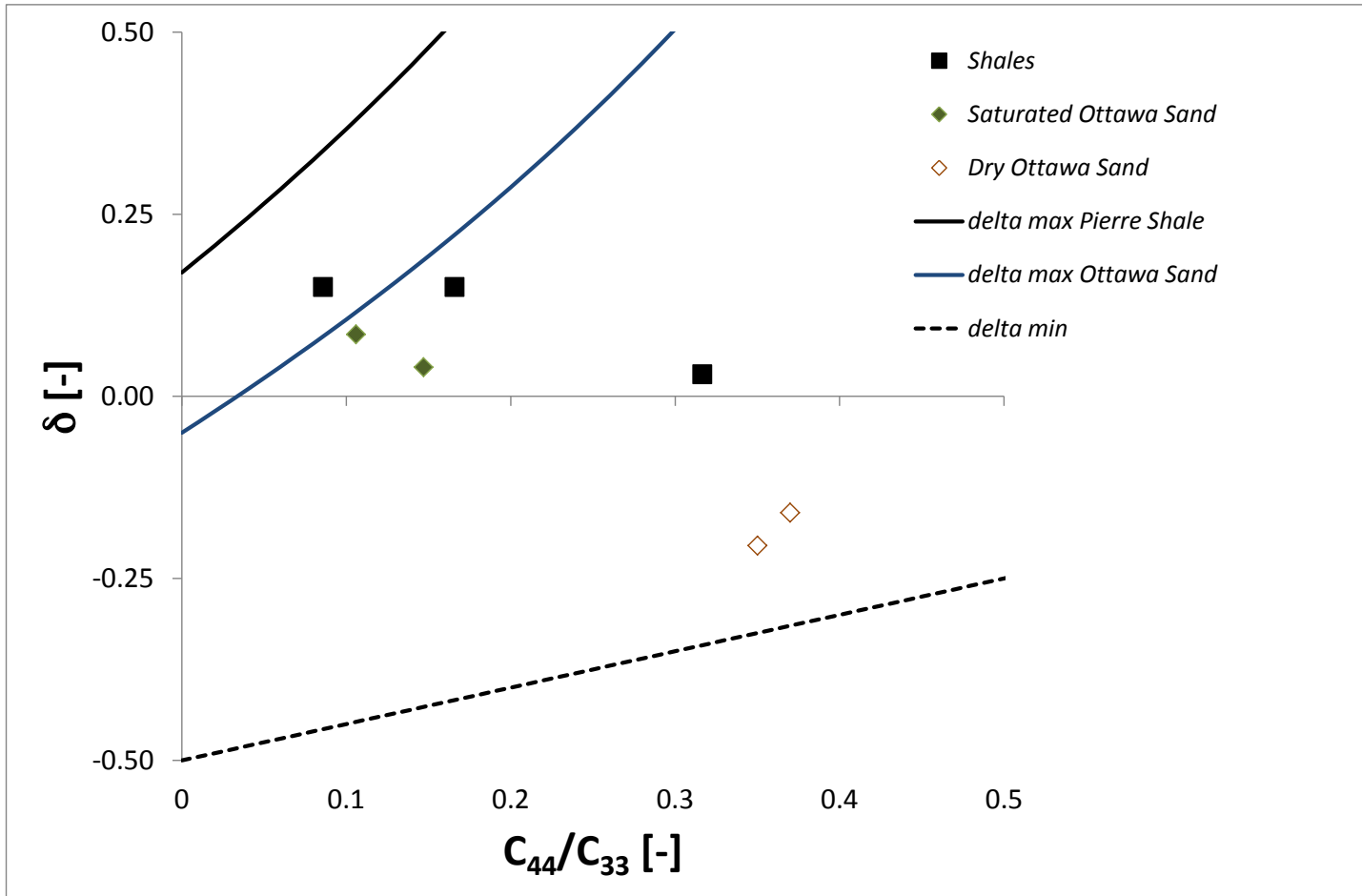
Mancos Shale: E-Modulus Anisotropy



Same parameters used in fit for P-wave modulus and E

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Measured δ vs. theoretical bounds



*Sand data from
Ph.D. work of
Bhuiyan (2014 + N*)*

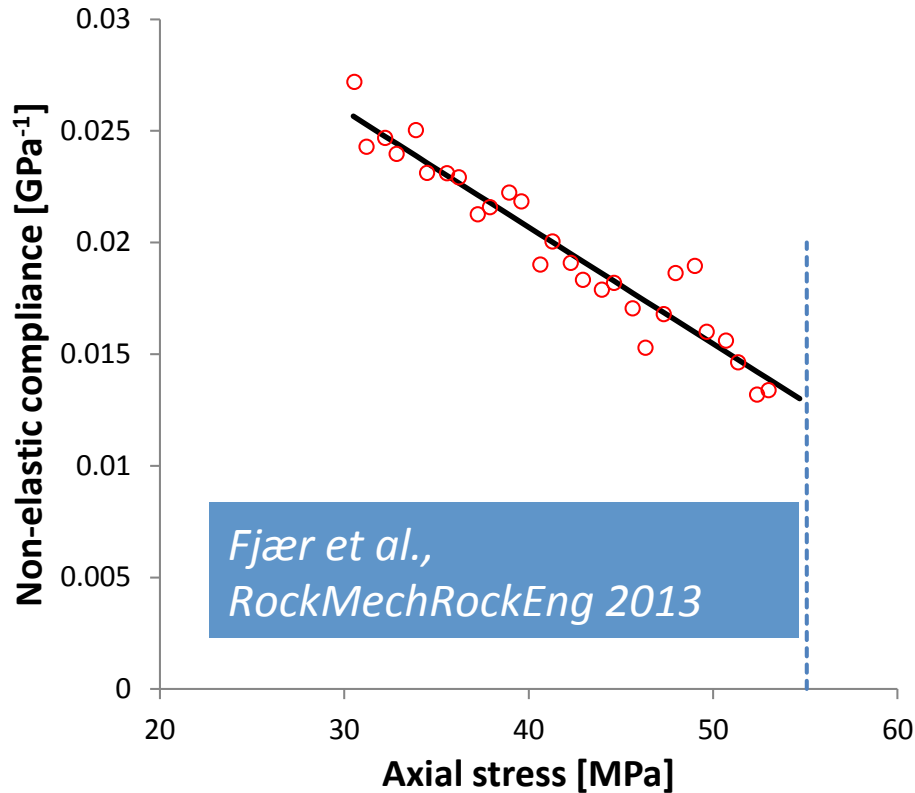
*N<1

Conclusions

- Dynamic E-moduli by far exceed static moduli in the shales studied, because of:
 - Non-elastic deformation
 - Dispersion
- Reasonable angular dependence of static E-modulus may be deduced from wave velocity anisotropy, using a fixed scaling parameter + dynamic anisotropy parameters
- C_{13} (or δ) is crucial for angular dependence
 - Bounds on C_{13} may be used to guide interpretation
 - For soft & saturated shale (+ sand) the maximum permitted value of δ may be <0

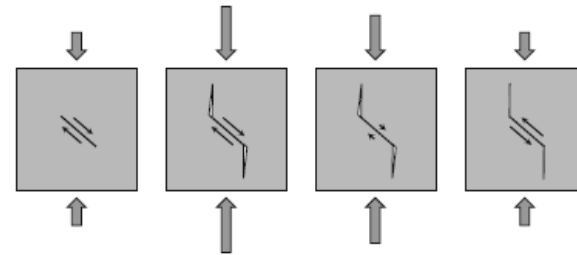
Acknowledgements to SIP on Gas Shale @ SINTEF
Petroleum Research + Statoil for funding of rock mechanics
tests + The Norwegian Research Council and industry for
support to ROSE @ NTNU and Shale Rock Physics @ SINTEF

Plasticity in Mancos Shale



$$\frac{1}{H_{non-el}} = \frac{1}{H_{stat}} - \frac{1}{(\rho v_P^2)}$$

The non-elastic compliance should disappear near turning points of the stress path, where the material behaves elastically



In a uniaxial strain experiment, static modulus corresponds to P-wave modulus

Non-zero non-elastic compliance when extrapolating to the turning point represents dispersion between quasi-static and ultrasonic frequency

Here: Estimated dispersion 20 %