

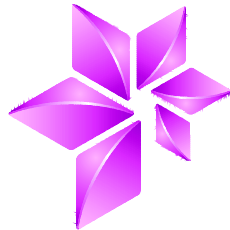
# Mindlin's friction term and implication for shear modulus and anisotropy in granular media

By

Kenneth Duffaut, Martin Landrø, Roger Sollie and Ørjan Pedersen

ROSE meeting, Trondheim

2. May, 2011



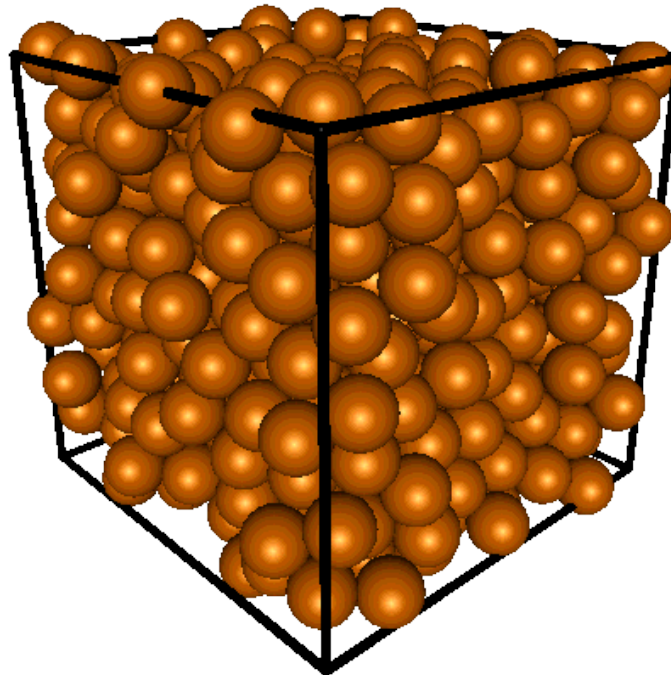
**Statoil**



**NTNU – Trondheim**  
Norwegian University of  
Science and Technology

# Objective

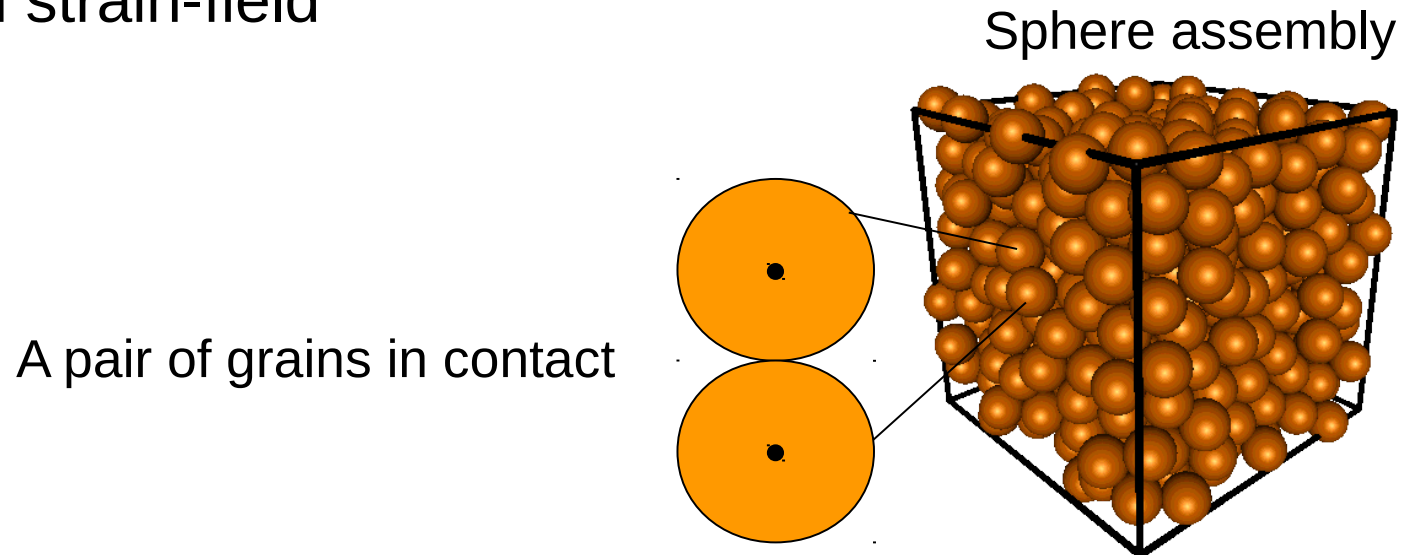
- Model effective elastic properties (e.g. shear rigidity) of a random packing of identical spheres with intergrain contact friction undergoing either hydrostatic or uniaxial loading



Sphere assembly

# Assumptions – Grain contact model

- Constant porosity or pore volume (~36%)
  - Random dense packing configuration
- Isotropic homogenous spherical grains
  - No variation with either position or direction
- Small strain (grain deformation  $\ll$  grain radius)
- Hydrostatic loading (stress equal in all directions)
- Mean strain-field



# Effective moduli – existing models

Effective bulk modulus

$$K_{\text{dry}} = \frac{C_p(1-\phi)}{12\pi R} S_N$$

Normal contact stiffness

$$S_N = \frac{\partial F_N}{\partial \delta_N} = \frac{4Ga}{(1-\nu)} \quad (\text{Hertz, 1882})$$

$S_T = 0$ : Effective shear modulus - ~small contact friction

$$G_{\text{dry}} = \frac{3}{5} K_{\text{dry}}$$

Tangential contact stiffness

$$S_T = \left( \frac{\partial F_T}{\partial \delta_T} \right) = 0, \quad \mu \approx 0$$

$S_T > 0$ : Effective shear modulus - ~infinite contact friction

$$G_{\text{dry}} = \frac{3}{5} \left( 1 + \frac{3}{2} \frac{S_T}{S_N} \right) K_{\text{dry}} = \frac{3}{5} \left( \frac{5-4\nu}{2-\nu} \right) K_{\text{dry}}$$

Tangential contact stiffness

$$S_T = \left( \frac{\partial F_T}{\partial \delta_T} \right) = \frac{8Ga}{(2-\nu)}, \quad \mu = \infty \quad (\text{Mindlin, 1949})$$

$F_N$  = Normal force

$\delta_N$  = Normal displacement

$a$  = Grain contact radius

$G$  = Grain shear modulus

$\nu$  = Grain Poisson's ratio

$R$  = grain radius

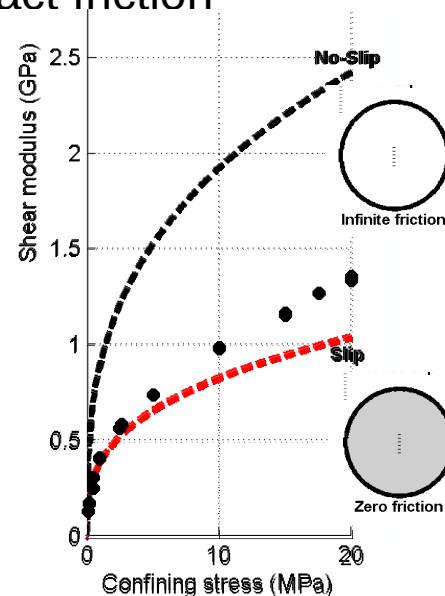
$C_p$  = coordination number

$\phi$  = Porosity

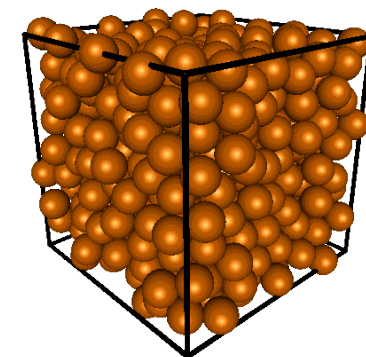
$F_T$  = Tangential force

$\delta_T$  = Tangential displacement

$\mu$  = Contact friction



Dry sphere assembly

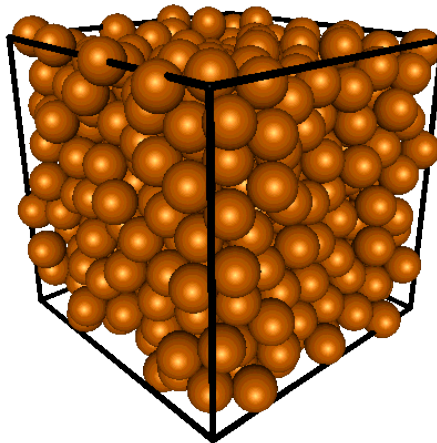
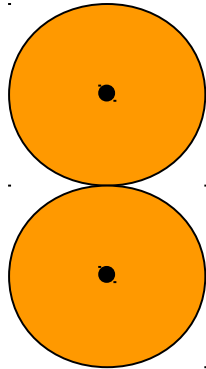


(Digby, 1981,  
Walton, 1987)

# Friction ( $\mu$ )

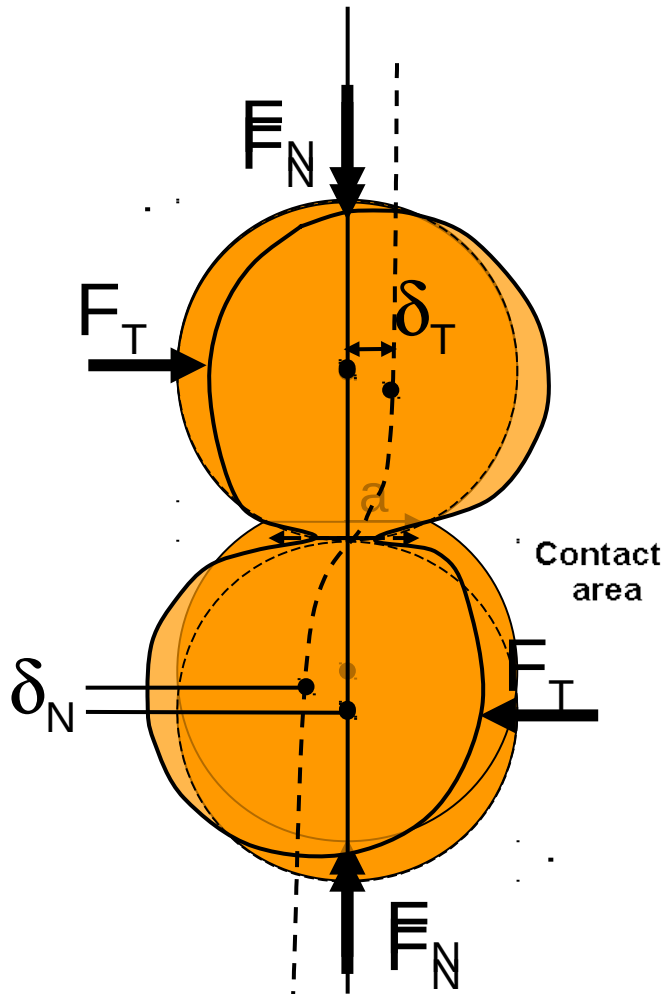
## Definition

- Friction is the force resisting the relative motion of two solid surfaces in contact
  - **Internal friction** is the force resisting motion between elements making up a solid while it undergoes deformation



# Contact stiffnesses - New model

A pair of identical grains with arbitrary contact friction



Schematic diagram

Normal contact stiffness

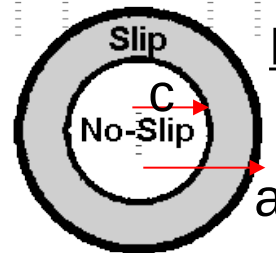
$$S_N = \frac{\partial F_N}{\partial \delta_N} = \frac{4Ga}{(1-\nu)}$$

Tangential contact stiffness

$$S_T = \left( \frac{\partial F_T}{\partial \delta_T} \right) = \frac{8Ga}{(2-\nu)} \left( 1 - \frac{F_T}{\mu F_N} \right)^{\frac{1}{3}} = \frac{8Ga}{(2-\nu)} f(\mu)$$

(Mindlin, 1949)

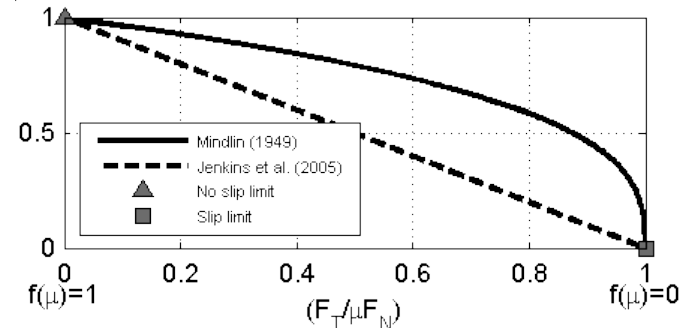
Partial slip



Bird view

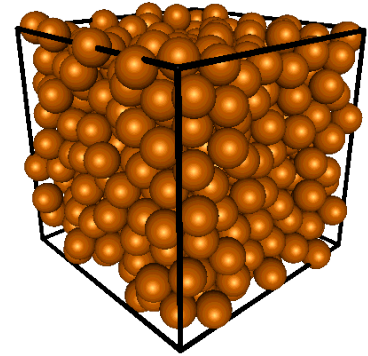
Non-zero friction

$$f(\mu) = \left( 1 - \frac{F_T}{\mu F_N} \right)^{\frac{1}{3}}$$



# Effective moduli – New model

Sphere assembly



$S_T = 0$ : Effective bulk and shear modulus - ~zero contact friction

$$K_{\text{dry}} = \frac{C_p(1-\phi)}{12\pi R} S_N$$

$$G_{\text{dry}} = \frac{3}{5} K_{\text{dry}}$$

$S_T > 0$ : Effective shear modulus - infinite contact friction

$$G_{\text{dry}} = \frac{3}{5} \left( 1 + \frac{3}{2} \frac{S_T}{S_N} \right) K_{\text{dry}} = \frac{3}{5} \left( \frac{5-4\nu}{2-\nu} \right) K_{\text{dry}}$$

(Digby, 1981,  
Walton, 1987)

$S_N$  = Normal contact stiffness

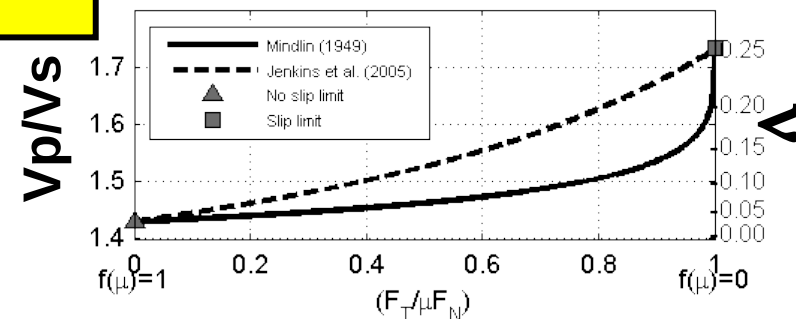
$S_T$  = Normal contact stiffness

$f(\mu)$  = Mindlin's friction term

$f(\mu) [0,1]$ : Effective shear modulus – arbitrary contact friction

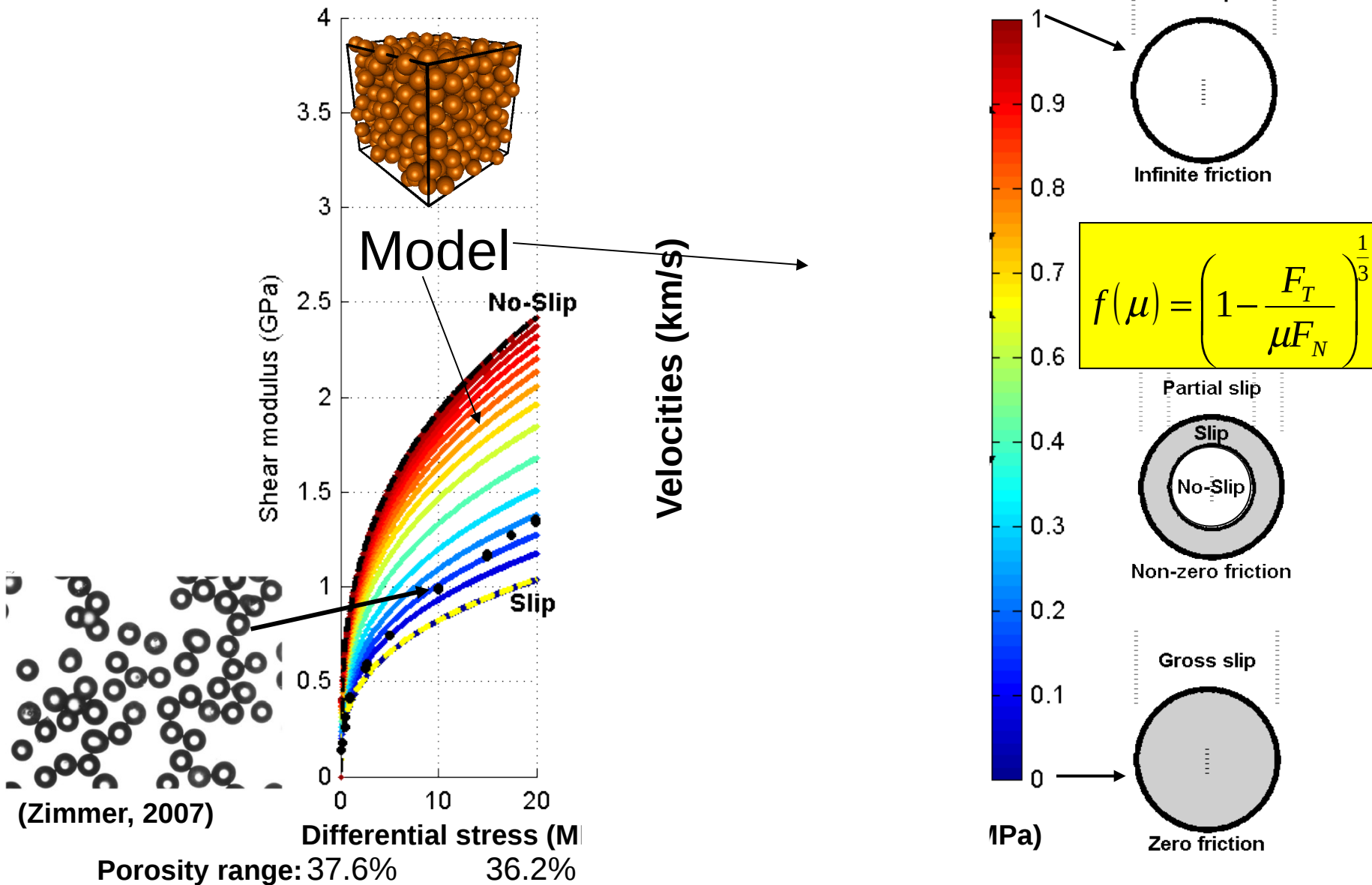
$$G_{\text{dry}} = \frac{3}{5} \left[ 1 + \frac{3}{2} \frac{S_T f(\mu)}{S_N} \right] K_{\text{dry}} = \frac{3}{5} \left[ 1 + \frac{3(1-\nu)}{2-\nu} f(\mu) \right] K_{\text{dry}}$$

$$f(\mu) = \left( 1 - \frac{F_T}{\mu F_N} \right)^{\frac{1}{3}}$$



# Model vs. dry core measurements

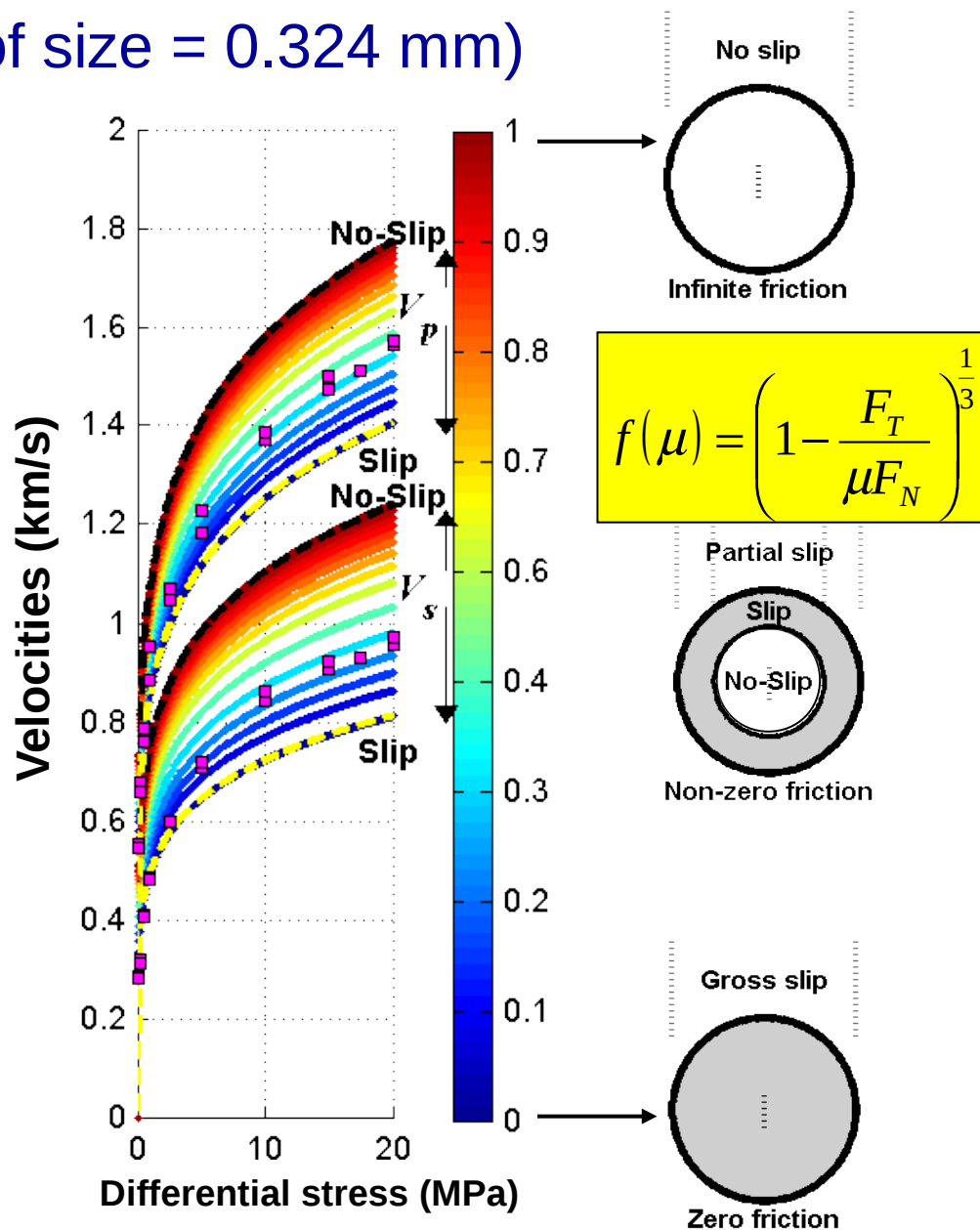
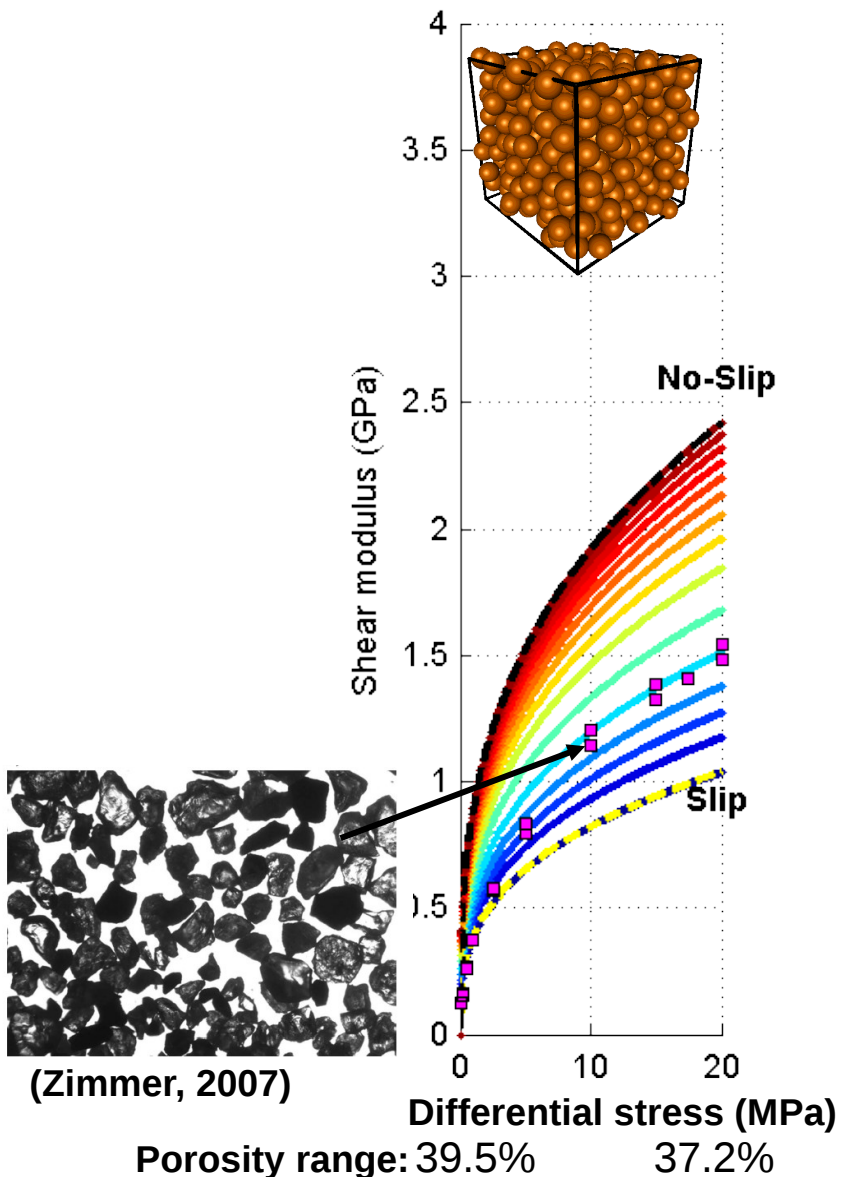
Loose glass beads (Spherical grains of size = 0.324 mm)





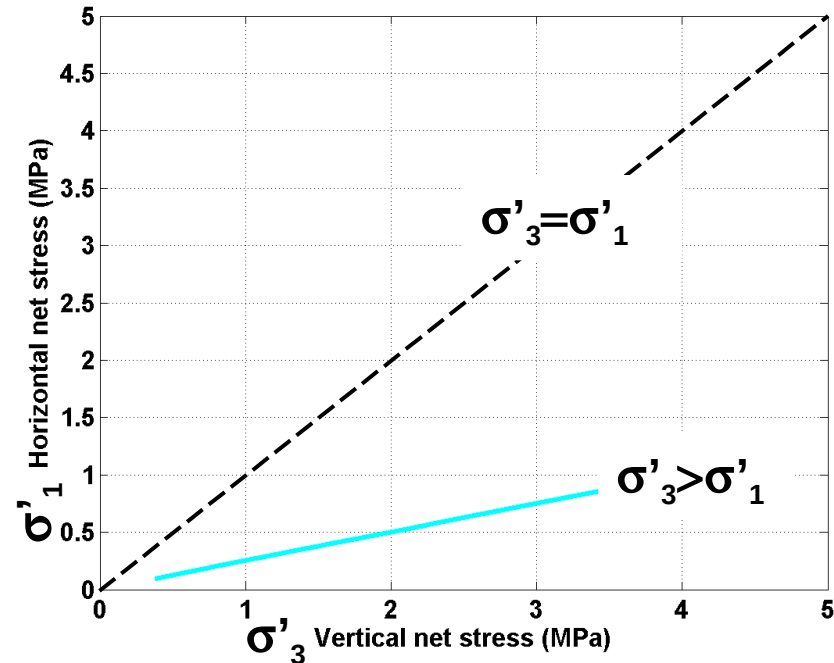
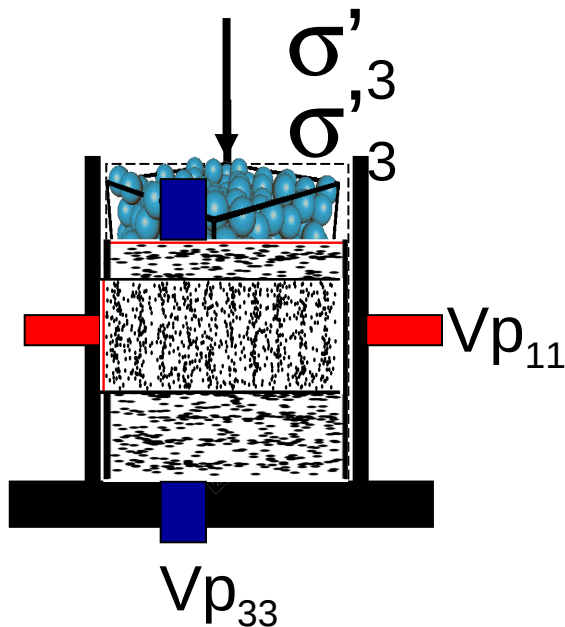
# Model vs. dry core measurements

Loose sand (angular grains of size = 0.324 mm)



# Uniaxial strain loading ( $e_3 \neq 0, e_1=e_2=0$ )

## Definition

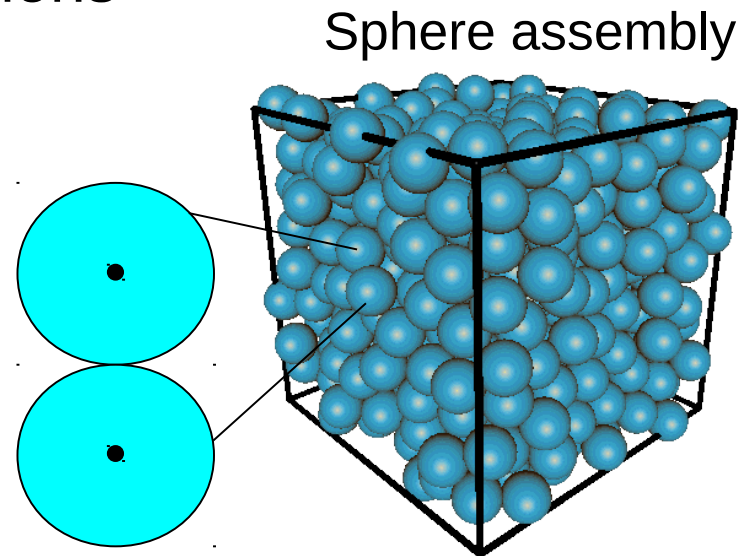


**Directional dependent velocity variation induced by stress differences**

# Assumptions – Grain contact model

- Constant porosity or pore volume (~36%)
  - Random dense packing configuration
- Isotropic homogenous spherical grains
  - No variation with either position or direction
- Small strain (grain deformation  $\ll$  grain radius)
- Uniaxial strain loading conditions
- Mean strain-field

A pair of grains in contact



# Walton model – infinite contact friction

Uniaxial strain loading – The sphere assembly is elastically anisotropic

Effective elastic stiffness constants:

$$C_{11}^{dry} = 3\alpha + 6\beta$$

$$C_{13}^{dry} = 2\alpha - 4\beta$$

$$C_{33}^{dry} = 8\alpha + 8\beta$$

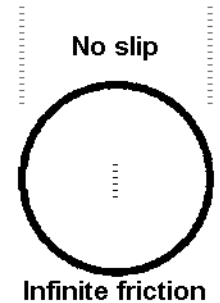
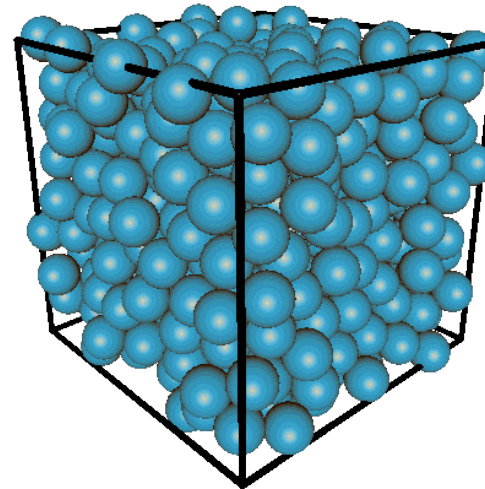
$$C_{44}^{dry} = C_{55}^{dry} = 2\alpha + 5\beta$$

$$C_{66}^{dry} = \alpha + 4\beta$$

where

$$\alpha = \frac{1}{4} \left( \frac{3(1-\phi)^2 C_p^2 G^2 (2-\nu) \sigma_3'}{16\pi^2 (1-\nu)^2 \nu} \right)^{\frac{1}{3}}$$

$$\beta = \frac{1}{4} \left( \frac{3(1-\phi)^2 C_p^2 G^2 (1-\nu) \sigma_3'}{16\pi^2 (2-\nu)^2 \nu} \right)^{\frac{1}{3}}$$



$\phi$  = Pore volume or Porosity

$\nu$  = Grain Poisson's ratio

$G$  = Grain shear modulus

$\sigma_3'$  = Vertical differential stress

$C_p$  = # grain contact points

# Walton model – ~ zero contact friction

Uniaxial strain loading – The sphere assembly is elastically anisotropic

Effective elastic stiffness constants:

$$C_{11}^{dry} = 3\alpha$$

$$C_{13}^{dry} = 2\alpha$$

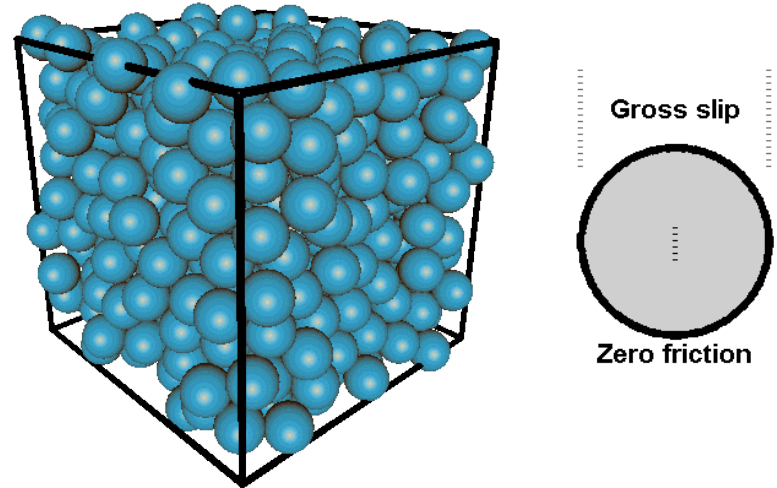
$$C_{33}^{dry} = 8\alpha$$

$$C_{44}^{dry} = C_{55}^{dry} = 2\alpha$$

$$C_{66}^{dry} = \alpha$$

where

$$\alpha = \frac{1}{4} \left( \frac{3(1-\phi)^2 C_p^2 G^2 (2-\nu) \sigma_3'}{16\pi^2 (1-\nu)^2 \nu} \right)^{\frac{1}{3}}$$



$\phi$  = Pore volume or Porosity

$\nu$  = Grain Poisson's ratio

$G$  = Grain shear modulus

$\sigma_3'$  = Vertical differential stress

$C_p$  = # grain contact points

# New model – Arbitrary contact friction

Uniaxial strain loading - Elastic stiffness constants as a function of  $S_N$ ,  $S_T$  and  $f(\mu)$

**Effective elastic stiffness constants:**

$$C_{11}^{\text{dry}} = \frac{3(1-\phi) C_p}{64 \pi R} [S_N + S_T f(\mu)]$$

$$C_{13}^{\text{dry}} = \frac{(1-\phi) C_p}{32 \pi R} [S_N - S_T f(\mu)]$$

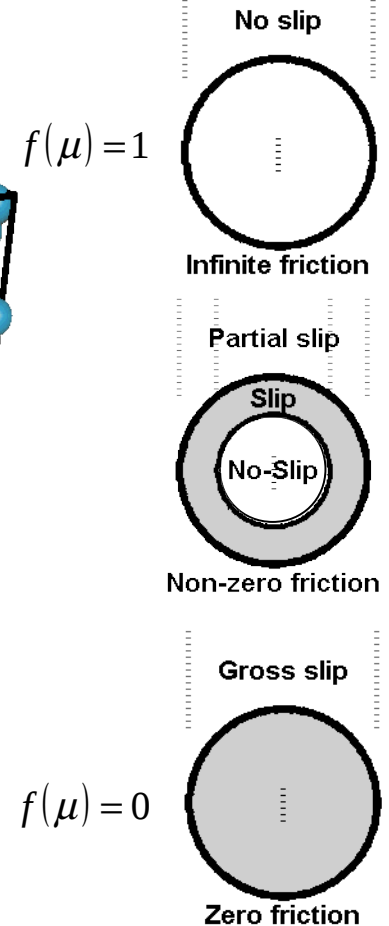
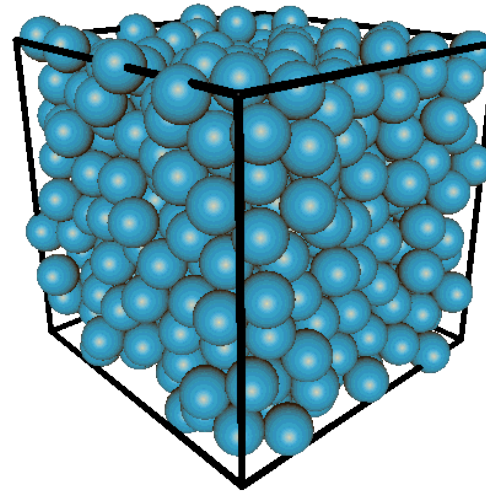
$$C_{33}^{\text{dry}} = \frac{(1-\phi) C_p}{16 \pi R} [2S_N + S_T f(\mu)]$$

$$C_{44}^{\text{dry}} = \frac{(1-\phi) C_p}{128 \pi R} [4S_N + 5S_T f(\mu)]$$

$$C_{66}^{\text{dry}} = \frac{(1-\phi) C_p}{64 \pi R} [S_N + 2S_T f(\mu)]$$

where

$$f(\mu) = \left(1 - \frac{F_T}{\mu F_N}\right)^{\frac{1}{3}}$$



$\phi$  = Pore volume or Porosity

$R$  = Grain radius

$C_p$  = # grain contact points

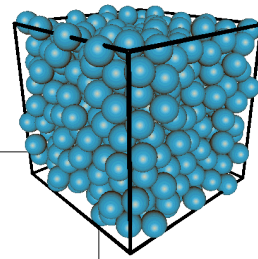
$S_N$  = Normal contact stiffness

$S_T$  = Tangential contact stiffness

$f(\mu)$  = Mindlin's friction term

# Model vs. dry loose sand measurements

## P-wave velocity vs. vertical net stress



### Vertical P-wave velocity:

$$V_{p_{33}}^{\text{dry}} = \sqrt{\frac{C_{33}^{\text{dry}}}{(1-\phi)\rho_{ma}}}$$



### Horizontal P-wave velocity:

$$V_{p_{11}}^{\text{dry}} = \sqrt{\frac{C_{11}^{\text{dry}}}{(1-\phi)\rho_{ma}}}$$

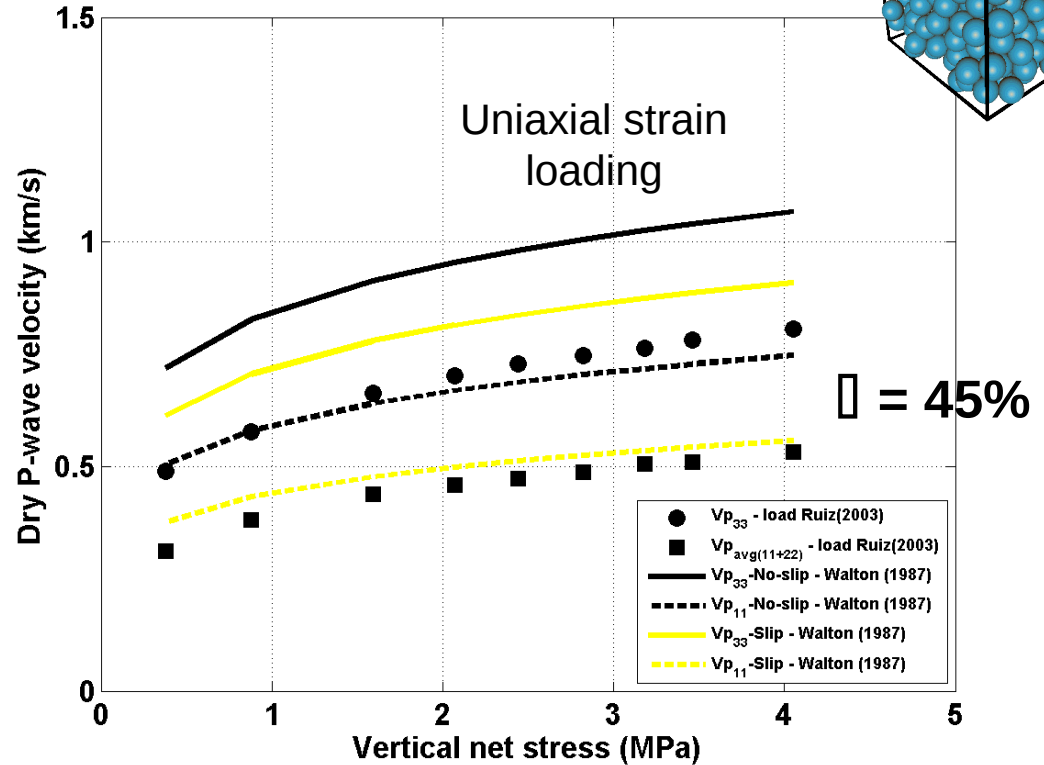


$\phi$  = Pore volume or Porosity

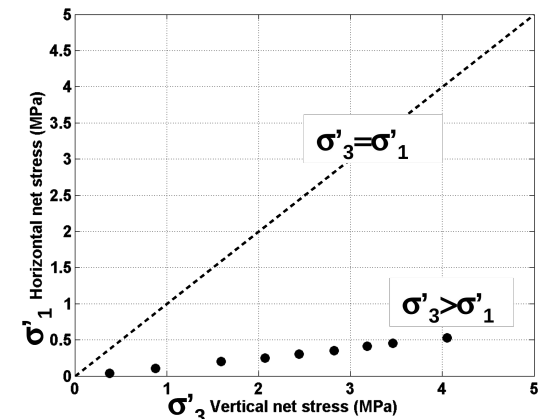
$\rho_{ma}$  = Grain density

$C_{33}^{\text{dry}}$  = Vertical P-wave modulus

$C_{11}^{\text{dry}}$  = Horizontal P-wave modulus



## Stress-induced velocity anisotropy



# New model

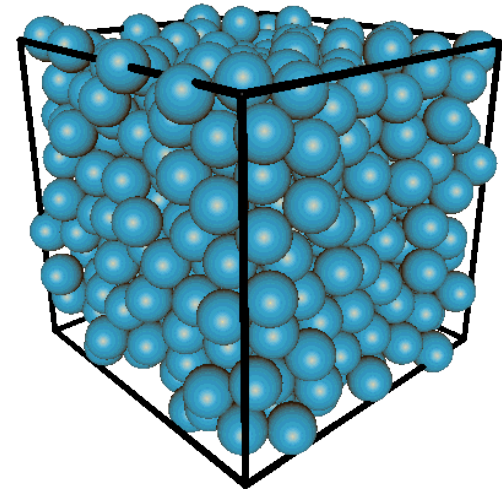
Thomsen<sup>(1986)</sup> anisotropy parameters as function of  $f(\mu)$  and  $\nu$

**P-wave anisotropy:**

$$\varepsilon = \frac{C_{11}^{\text{dry}} - C_{33}^{\text{dry}}}{2C_{33}^{\text{dry}}} = - \frac{\left[ 5 + 2 \left( \frac{1-\nu}{2-\nu} \right) f(\mu) \right]}{16 \left[ 1 + \left( \frac{1-\nu}{2-\nu} \right) f(\mu) \right]}$$

**S-wave anisotropy:**

$$\gamma = \frac{C_{66}^{\text{dry}} - C_{44}^{\text{dry}}}{2C_{44}^{\text{dry}}} = - \frac{\left[ 1 + \left( \frac{1-\nu}{2-\nu} \right) f(\mu) \right]}{2 \left[ 2 + 5 \left( \frac{1-\nu}{2-\nu} \right) f(\mu) \right]}$$



$$\delta = \frac{(C_{13}^{\text{dry}} + C_{44}^{\text{dry}})^2 - (C_{33}^{\text{dry}} - C_{44}^{\text{dry}})^2}{2C_{33}^{\text{dry}}(C_{33}^{\text{dry}} - C_{44}^{\text{dry}})} = - \frac{9 \left[ 2 + \left( \frac{1-\nu}{2-\nu} \right) f(\mu) \right]^2 - \left[ 4 + \left( \frac{1-\nu}{2-\nu} \right) f(\mu) \right]^2}{48 \left[ 1 + \left( \frac{1-\nu}{2-\nu} \right) f(\mu) \right] \left[ 2 + \left( \frac{1-\nu}{2-\nu} \right) f(\mu) \right]}$$

$\nu$  = Grain Poisson's ratio

$f(\mu)$  = Mindlin's friction term

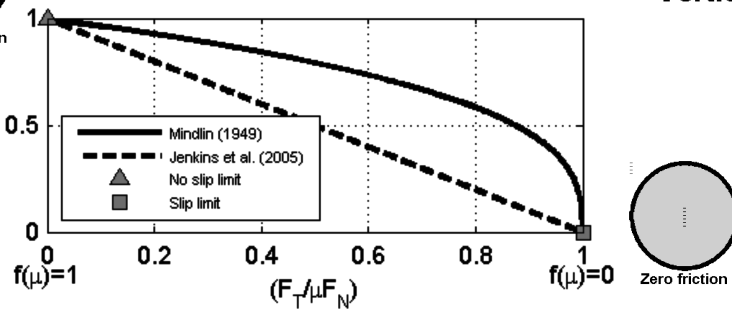
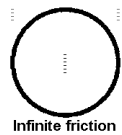
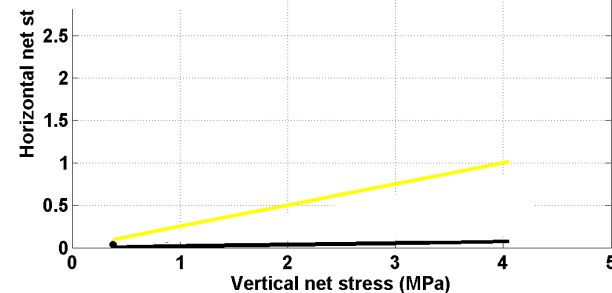
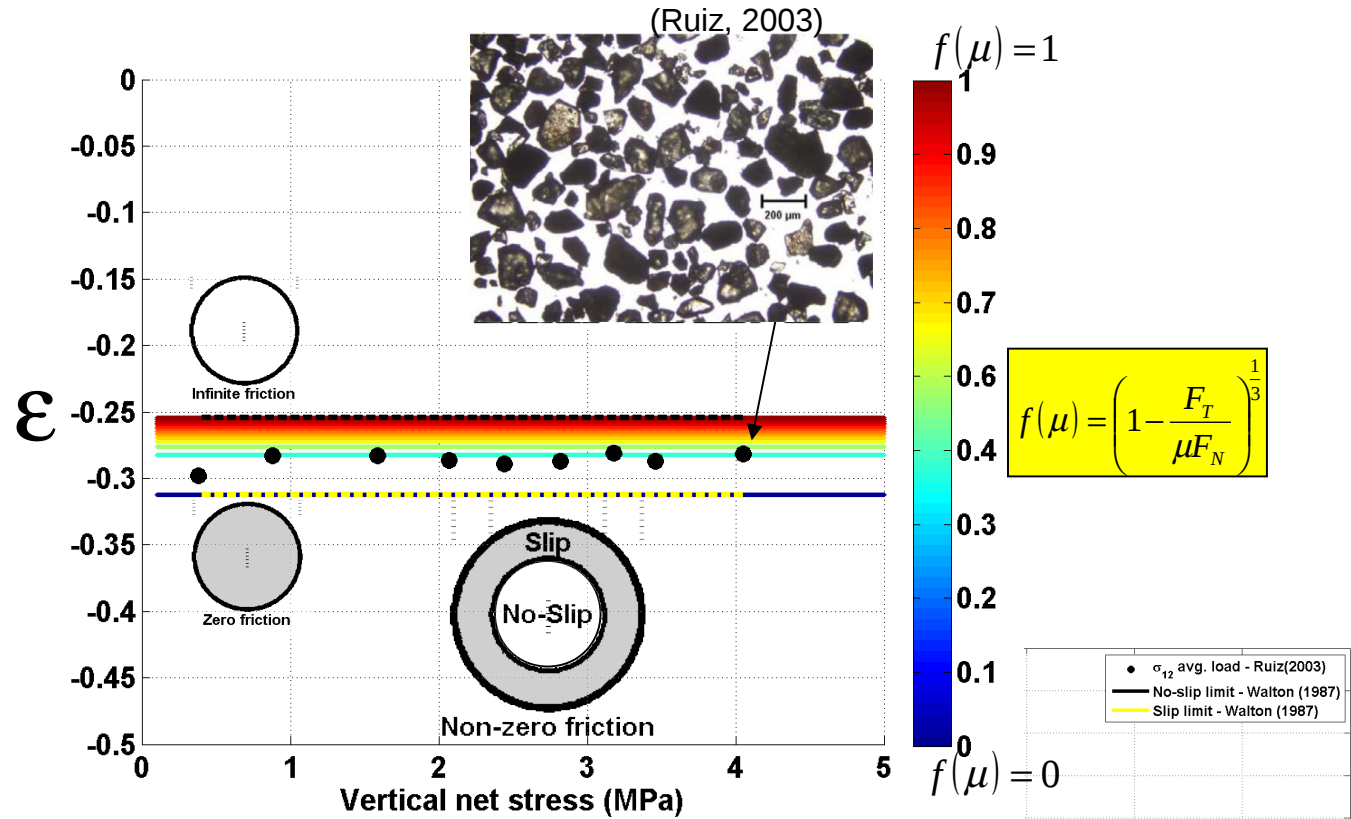


# Model vs. dry loose sand measurements

## Stress- and friction-induced elastic anisotropy

P-wave anisotropy:

$$\varepsilon = - \frac{\left[ 5 + 2 \left( \frac{1-\nu}{2-\nu} \right) f(\mu) \right]}{16 \left[ 1 + \left( \frac{1-\nu}{2-\nu} \right) f(\mu) \right]}$$

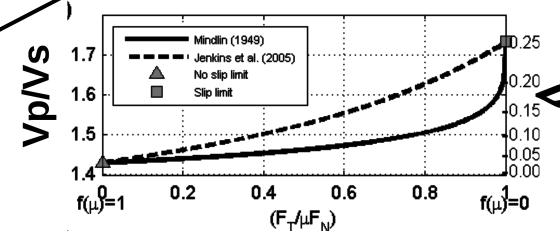
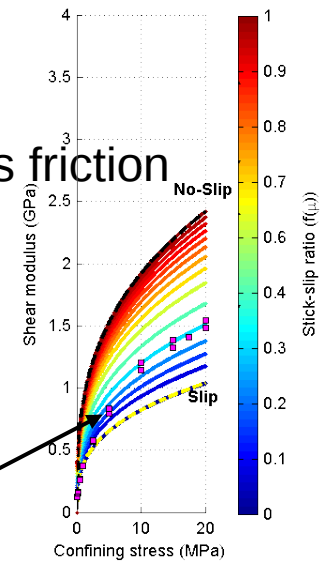


# Conclusions

- The shear modulus is made friction-dependent by use of Mindlin's friction theory

$$G_{\text{dry}} = \frac{3}{5} \left[ 1 + \frac{3(1-\nu)}{2-\nu} f(\mu) \right] K_{\text{dry}}$$

- As intergrain friction increases
  - Larger effective moduli and velocities and lower  $V_p/V_s$
  - Stress-sensitivity
    - increases for moduli (e.g.  $dG/d\sigma'$ ) and velocities ( $dV/d\sigma'$ )
    - decreases for the  $V_p/V_s$



- Ultrasonic measurements on sand of angular grains show higher dynamic shear rigidity than the perfect slip model – Increasing internal friction due to grain interlocking
- Grain contact conditions are controlling velocities and their stress-sensitivity in loose sands

# Conclusions cont'

- Mindlin's friction theory revisited for uniaxial loading conditions – The friction-dependent model predicts
  1. instant large negative stress-induced elastic anisotropy (>20%)
  2. 20% difference in stress-induced elastic anisotropy between small and infinite contact friction
    - Extrapolating observation to cemented sandstone -> less stress-induced elastic anisotropy
  3. The model is only valid for small stress ratios  $\leq 1/4$
- Ultrasonic measurements on a set of loose sand are in accordance with model-predictions
  - The sand with angular grains has higher internal friction than that of perfectly smooth spheres

Thank you